

Using BrR To Evaluate Bascule Bridges

2018 RADBUG MEETING – BOISE, IDAHO
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Presenters:

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A Joint Venture Teaming of Alfred Benesch & Company and Collins Engineers, Inc.

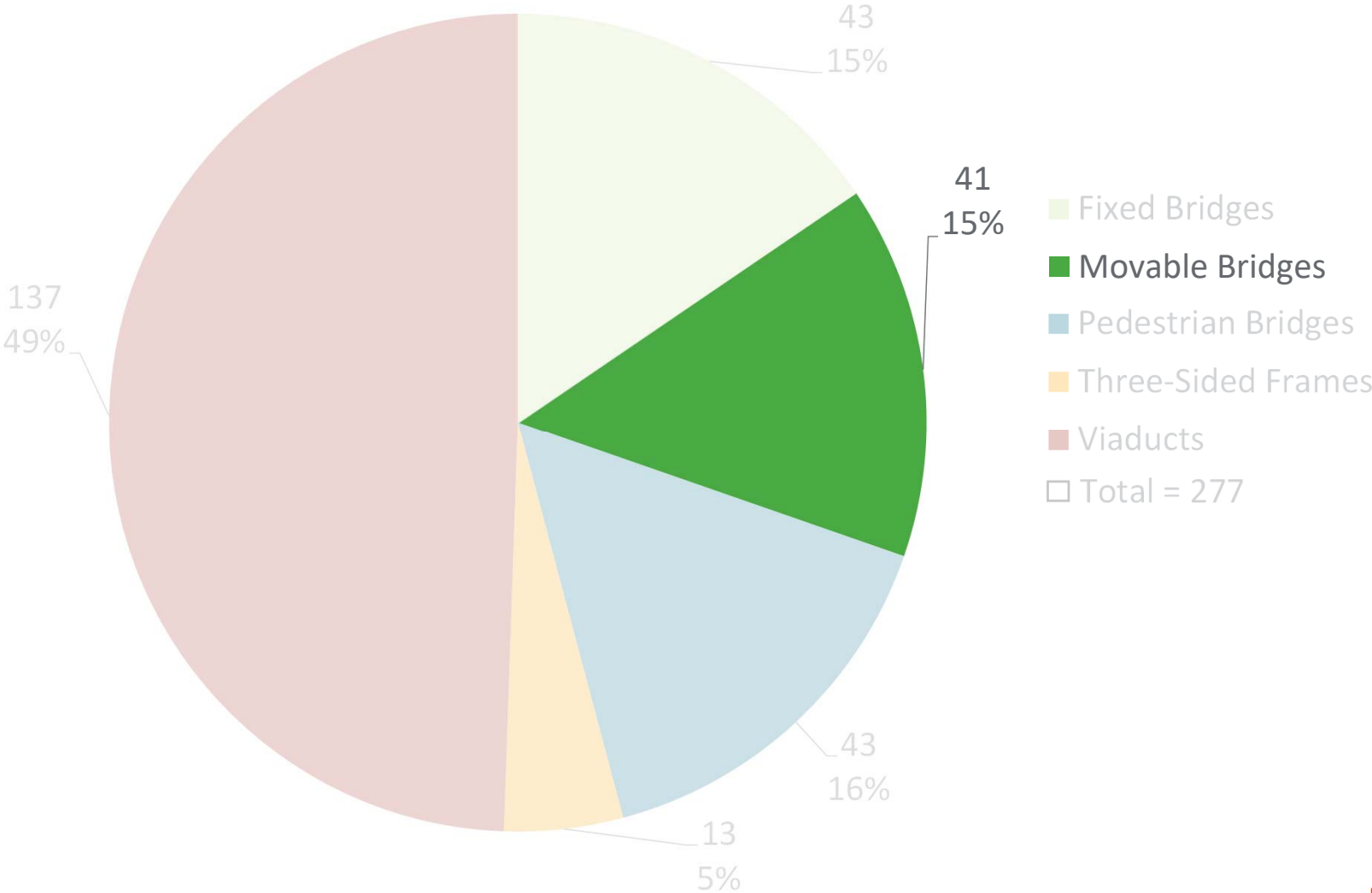


Using BrR To Evaluate Bascule Bridges

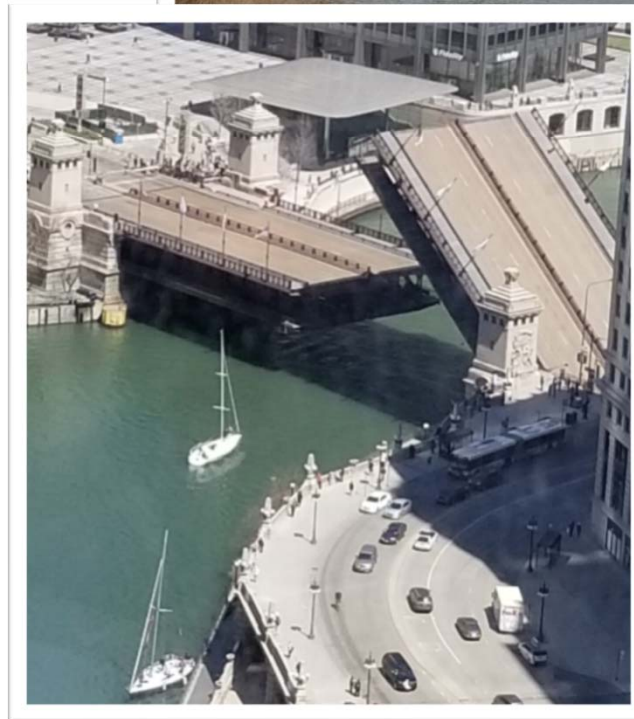
Summary of Presentation

- Background on CDOT & CBIT
- LaSalle St. and Lake St. Bascule Bridges – Carolyn Kois, P.E.
- Roosevelt Rd. and Cermak Rd. Bascule Bridges – Jim Surber, P.E, S.E.

Five Bridge Types Maintained by



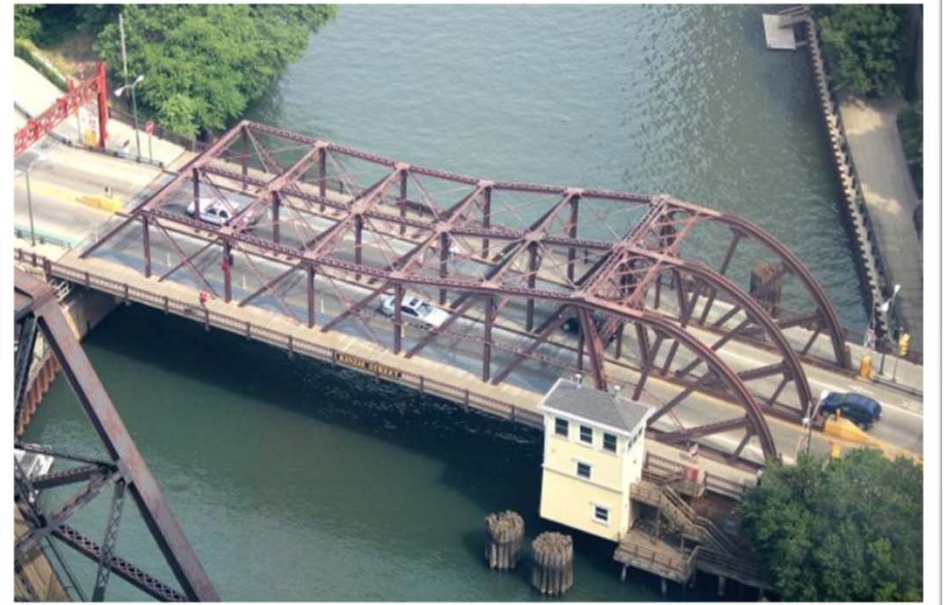
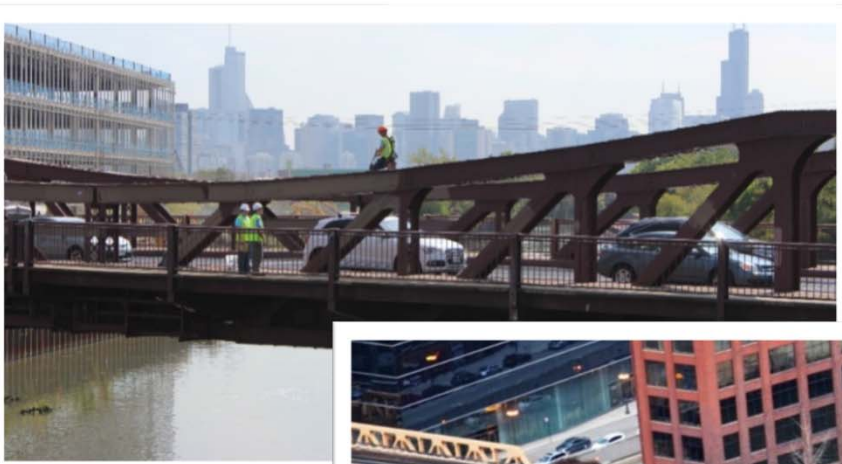
Chicago Bascule Bridges



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Chicago Bascule Bridges



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Who is CBIT?

Chicago
Bridge
Inspection
Team



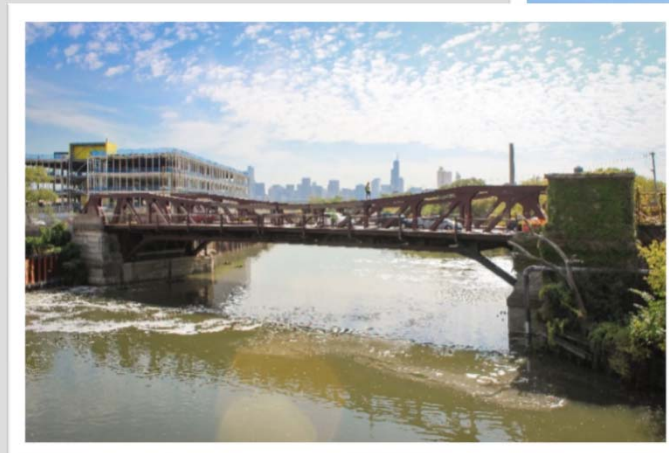
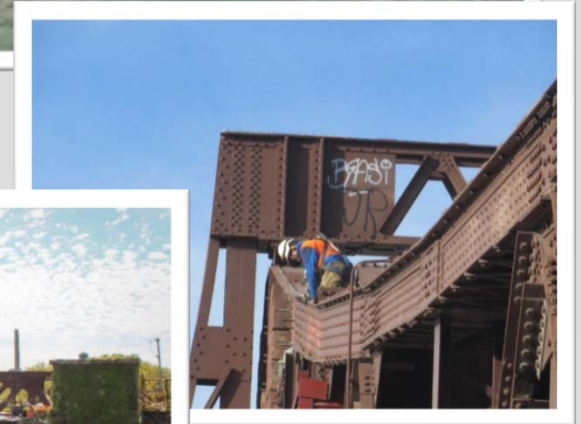
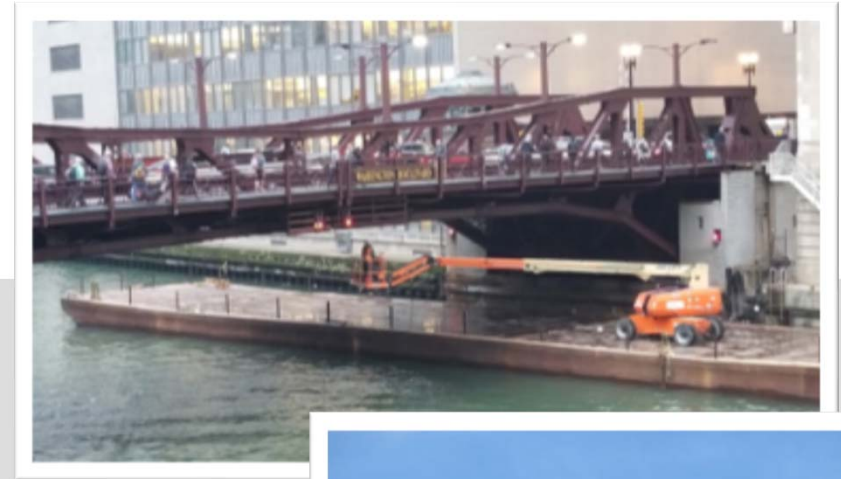
A Joint Venture Teaming of Alfred Benesch
& Company and Collins Engineers, Inc.





A Joint Venture Teaming of Alfred Benesch & Company and Collins Engineers, Inc.

- Bridge Inspections
 - Routine, Special, Fracture Critical, Underwater and Element Level
 - Mechanical and Electrical of movable bridges
 - NDT including UT testing
 - Confined spaces
 - Rope access, barges and man-lifts
- On-Call Engineering Services
 - Bridge repairs
 - Emergency services
 - Bascule bridge balancing
- Load Rating



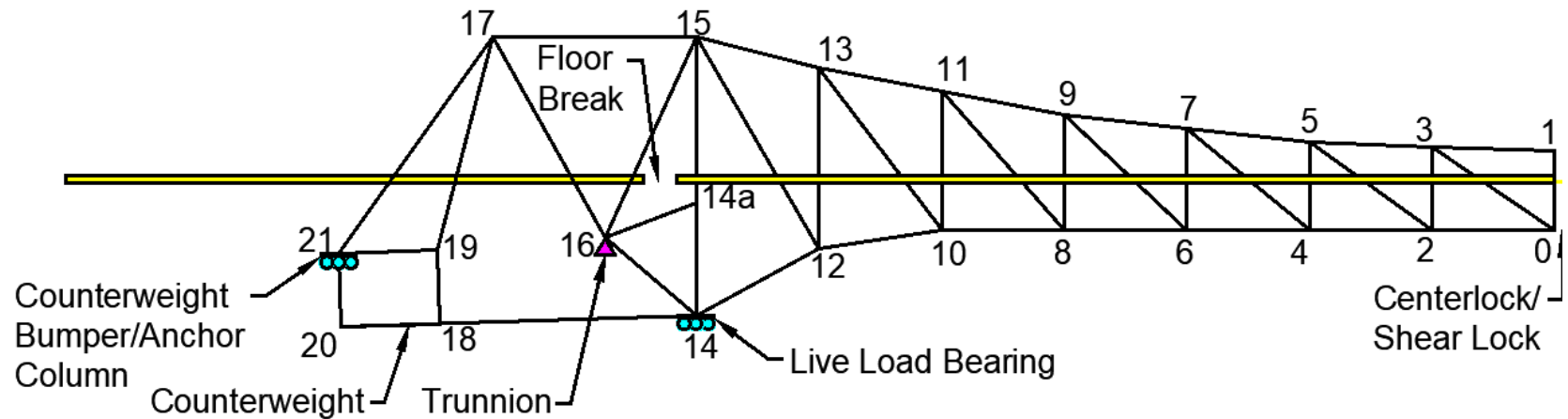
LaSalle St. Bascule

Single Level Double Leaf Trunnion Type Bascule



LaSalle St. Bascule

Bascule Components and Supports



LaSalle St. Bascule

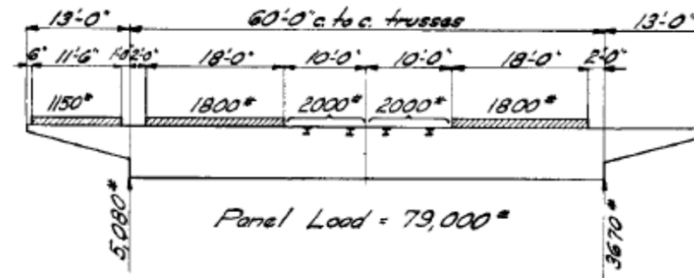
Original Construction Construction/Repair History

- 1928 Original Construction
- 1969 Timber Deck Replaced with Steel/Concrete Deck rebalancing of counterweight

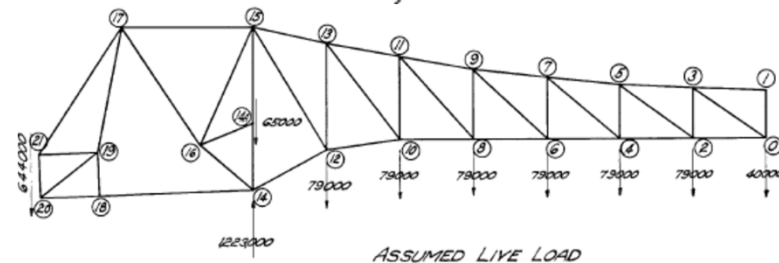


Historical Photo of La Salle Street Bridge
Source: Armour Engineer, Volume 22, No. 3, 1931

Design Load



CONDITION OF MAXIMUM LIVE LOADING OF TRUSSES:
Loads shown are per lineal foot.



Geometry

- 219'-10" between live load bearings
- 11'-6" live load bearings to trunnions
- 35"-3" trunnions to anchor columns

LaSalle St. Bascule

Carrying Elements

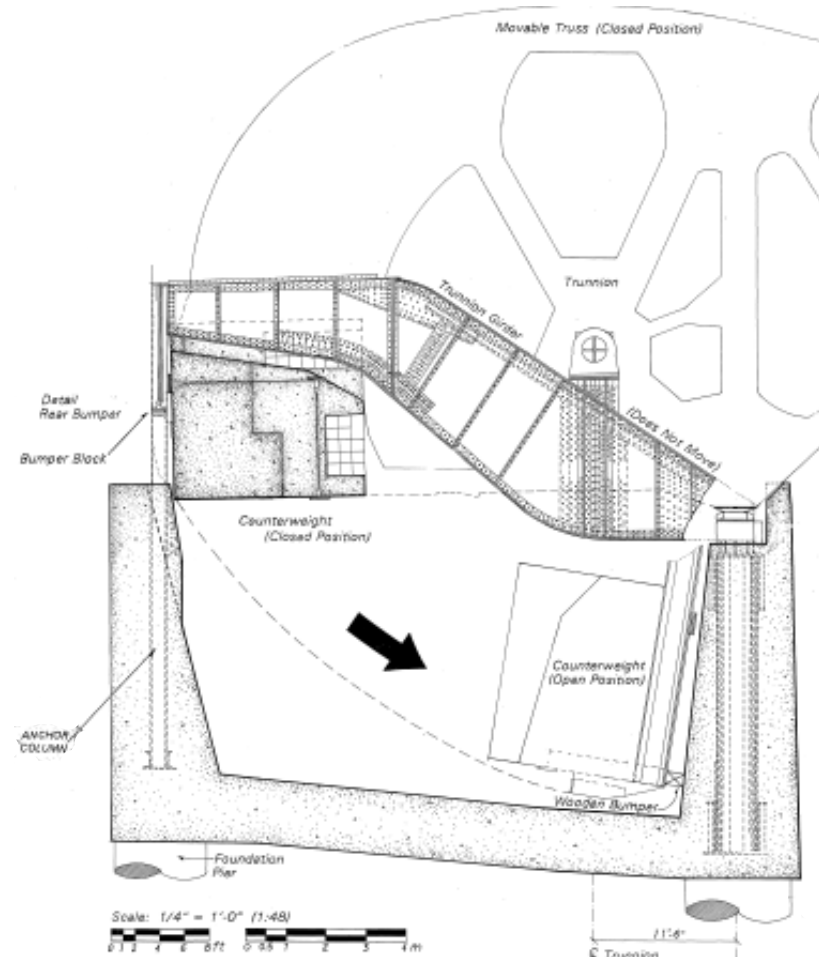
- Fixed span elements mostly rated in BrR
- Supplemental analysis required outside of BrR for some carrying elements



The concrete substructure of the bridge rests on massive concrete piers that are built down to bed rock.

Construction of Bridge

Source: Armour Engineer, Volume 22, No. 3, 1931



Section Through Counterweight Pit Showing Structural Detail

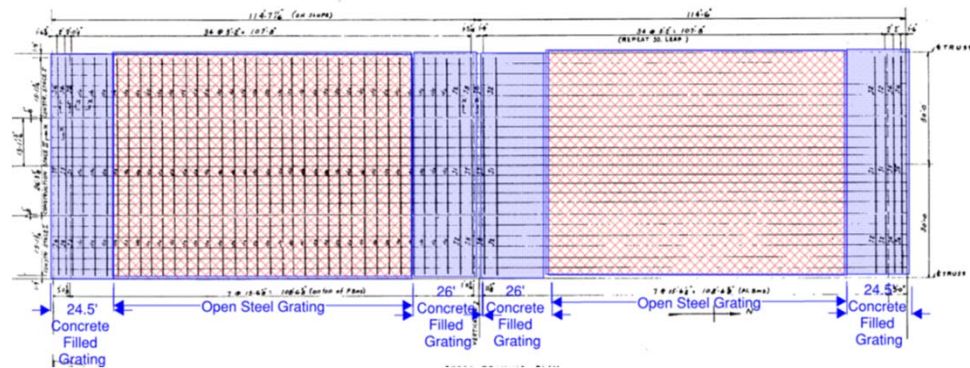
LaSalle St. Bascule

Dead Load Forces

Dead Loads

- Truss member self weight
- Floorbeams
- Stringers
- Cantilever sidewalks
- Lateral Bracing
- Deck
- Counterweight

Steel Grating Deck with Concrete Infill at Centerlock and Floorbreaks



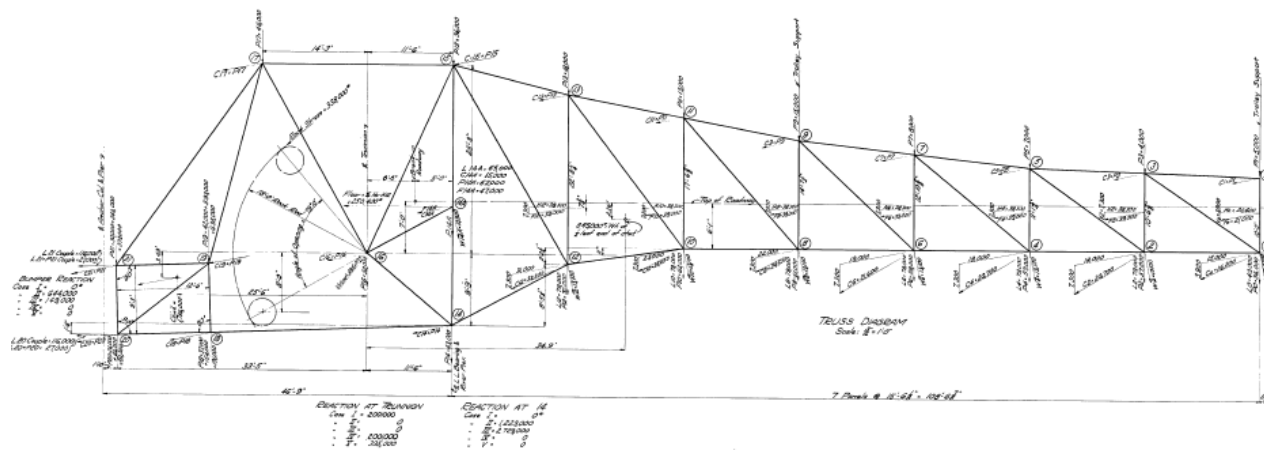
- Non-composite deck
- Deck thickness/weight adjusted to reflect weight of open steel grating
- Additional weight of concrete fill applied as distributed loads

LaSalle St. Bascule

Dead Load Balancing

Balanced Condition

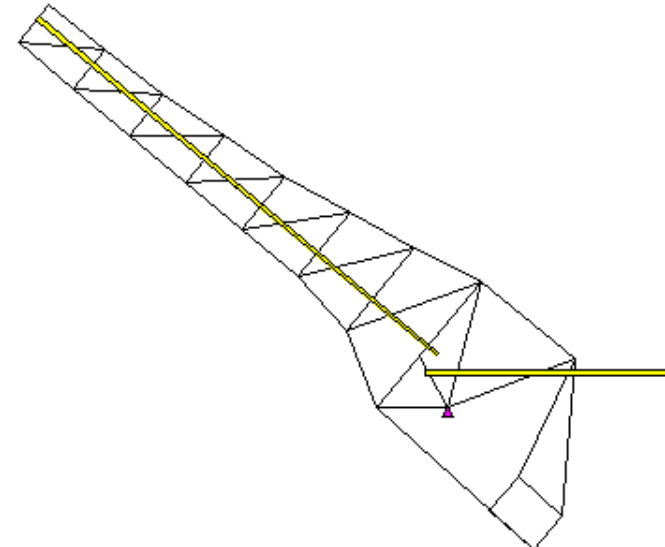
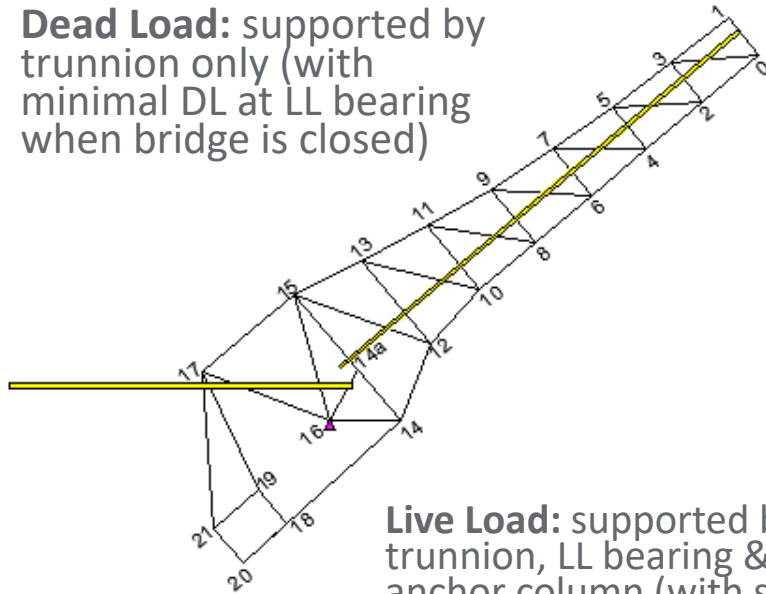
- Counterweight for DL model based on design and rehab plans
- Bascules rebalanced after repairs/alterations/maintenance
- Reaction at live load bearing is calculated for balanced condition (slightly nose-heavy)
- Counterweight is verified by comparing DL reaction at LL bearing reaction based on plan weight to LL bearing reaction calculated for balanced condition



LaSalle St. Bascule

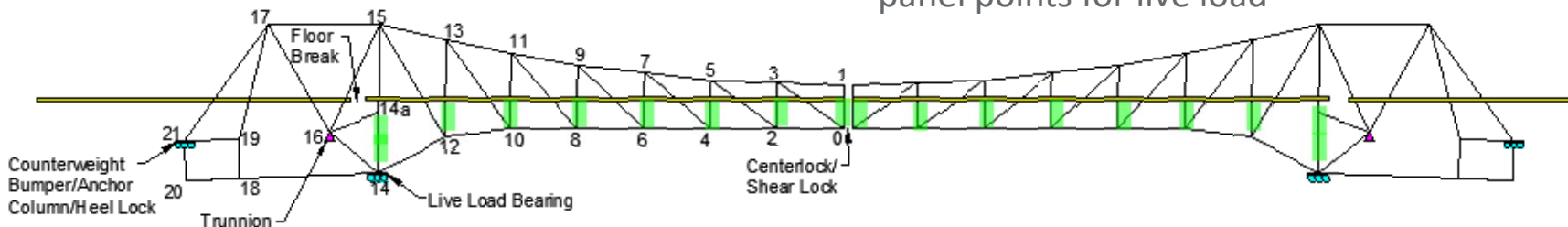
Load Stages

Dead Load: supported by trunnion only (with minimal DL at LL bearing when bridge is closed)



Live Load: supported by trunnion, LL bearing & anchor column (with shear transfer at centerlock)

■ Half deck line locations in BrR set to reflect the loaded panel points for live load



Pedestrian load included with live load

LaSalle St. Bascule

Live Load

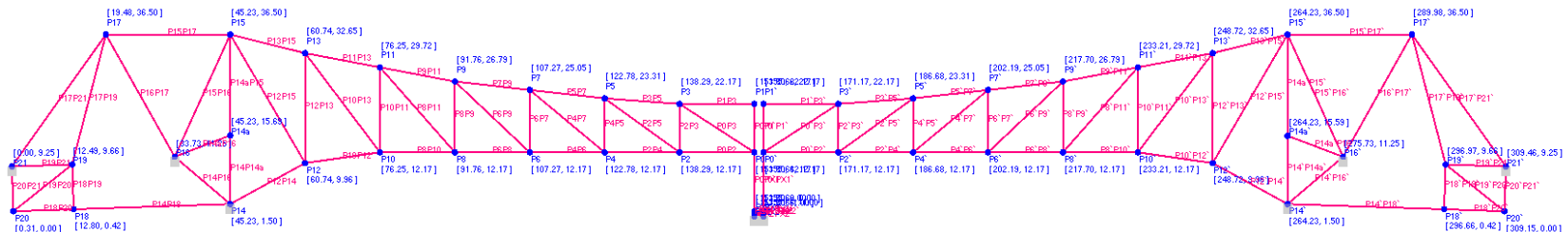
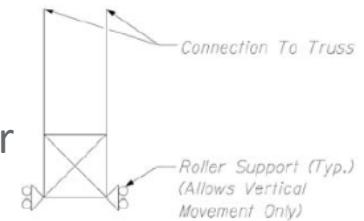
Vehicle and Pedestrian Load

Supports

- Pin Support at Trunnion
- Roller Support at LL Bearing
- Roller Support at CW Bumper

Centerlock

- Centerlock transfers shear
- Does not transfer moment or axial load



LaSalle St. Bascule

Verification of Dead Load Support Conditions

Location	Design Stress Sheet DL (k)	AASHTOWare Member	STAAD DL balance model (no centerlock or heel lock)	Location	Design Stress Sheet DL (k)	AASHTOWare Member	STAAD DL balance model (no centerlock or heel lock)
Top Chord		1-3	0	Diagonals	-71	0-3	-87
	-59	3-5	-72		-171	2-5	-214
	-198	5-7	-247		-225	4-7	-243
	-372	7-9	-434		-278	6-9	-277
	-582	9-11	-643		-266	8-11	-239
	-762	11-13	-803		-153	10-13	-113
	-866	13-15	-883		176	12-15	204
	-1160	15-17	-1128		-730	14-14a	-738
Bottom Chord	59	0-2	71	Rear Members	-801	14a-15	-752
	198	2-4	245			14a-16	0
	371	4-6	431		454	14-16	477
	572	6-8	631		405	14-18	393
	749	8-10	788		412	18-20	399
	848	10-12	865		984	15-16	910
856	12-14	862	1537		16-17	1506	
Verticals	5	0-1	2		-1179	17-19	-1176
	42	2-3	50				
	87	4-5	105				
	132	6-7	141				
	138	8-9	128				
	179	10-11	160				
	73	12-13	39				

Note: Numerous structure components, including the deck, have been modified since original construction. Original dead load forces are provided for information, however the dead load used for analysis was calculated based on the current structure geometry.

Location	Member	Bridge Closed					Bridge Opening or Fully Opened										
		Dead Load	Live Load No Shear Lock	Live Load From Shear Lock	Impact Coefficient	Impact	Col. 'A' Combined Stress Without Reversal	50% Reversal	Total Combined Stress	D.L. Closed	D.L. Open	20% Vib	20% Wind	Col. 'B' Combined Stress Without Reversal	50% Reversal	Total Combined Stress	
1-3																	
3-5		-59.0	-59.0	-124.0	$\frac{100}{(7.75 \times 12.5) + 300} = 0.97$	-18.0	-260.0	± 49.0	-309.0	-59.0	-2.0	-12.0	$\neq 11.0$	-71.0	± 4.0	-75.0	
5-7		-198.0	-221.0	-149.0	$\frac{100}{(7.75 \times 33.0) + 300} = 1.08$	-33.0	-601.0	± 17.0	-618.0	-198.0	-22.0	-4.0	$\neq 39.0$	-238.0	± 8.0	-246.0	
					$\frac{100}{(7.75 \times 7.5) + 300} = 1.21$	+18.0	+35.0	± 52.0									



LaSalle St. Bascule

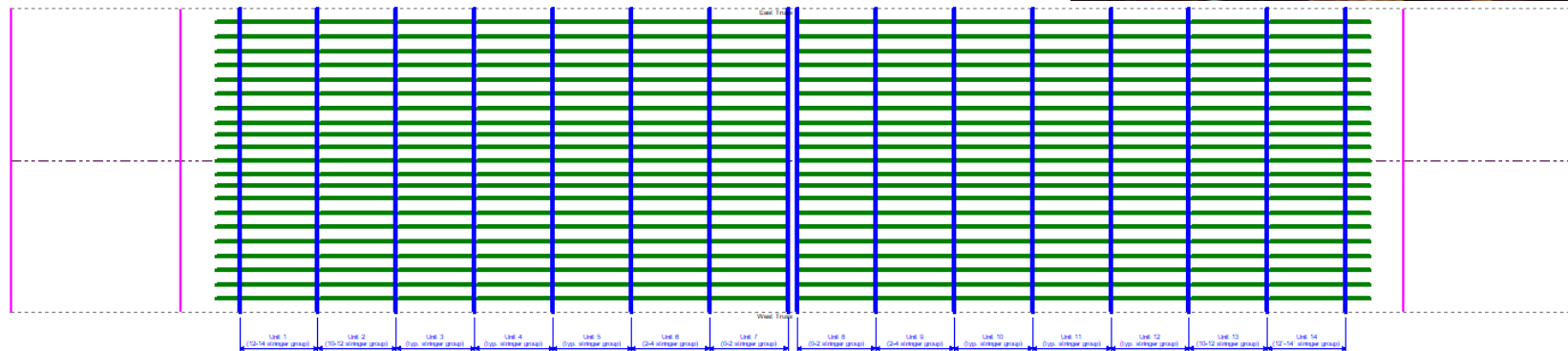
Load Rating Analysis

Floorbeams and Stringers

- Moveable portion modeled in BrR in truss superstructure definition
- Fixed spans modeled in BrR as separate superstructure alternative



Note: BrR does not model simple span floorbeams when there are more than 2 truss lines in a bridge. Floorbeams are analyzed as continuous so workaround is required to model them as simple span between trusses.



Lake St. Bascule

Double Level Double Leaf Trunnion Type Bascule



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Lake St. Bascule

Original Construction

Construction/Repair History

- 1914 Original Construction
- 1994 lower level floor system replacement and rebalancing of counterweight
- Various other repairs over the history of the bridge

Geometry

- 245'-3" between live load bearings
- 14'-0" live load bearings to trunnions
- 37"-7" trunnions to heel locks



OLD AND NEW DRAWBRIDGES OVER THE CHICAGO RIVER AT LAKE ST.

Design Load

Live load for floor system, hangers and posts, a floor load of 100 per square foot of sidewalk and roadway combined with two 50 ton electric cars on each track, or with one 24 ton truck on any part of the roadway*

25000* 25000*	25000* 25000* 25000* 25000*	25000* 25000*	24000* 24000*
○ 7ft ○	○ 7ft ○ 8ft ○ 7ft ○	○ 7ft ○	○ 10ft ○
26 ft	26 ft		Wheels 5'-0" apart
2-50 Ton Cars			24 Ton Truck

Space occupied by above loadings assumed as 10 ft in width

Live load for trusses, 2000 per lineal foot of each track, and 100* per square foot of sidewalk and roadway surface not occupied by the track loads*

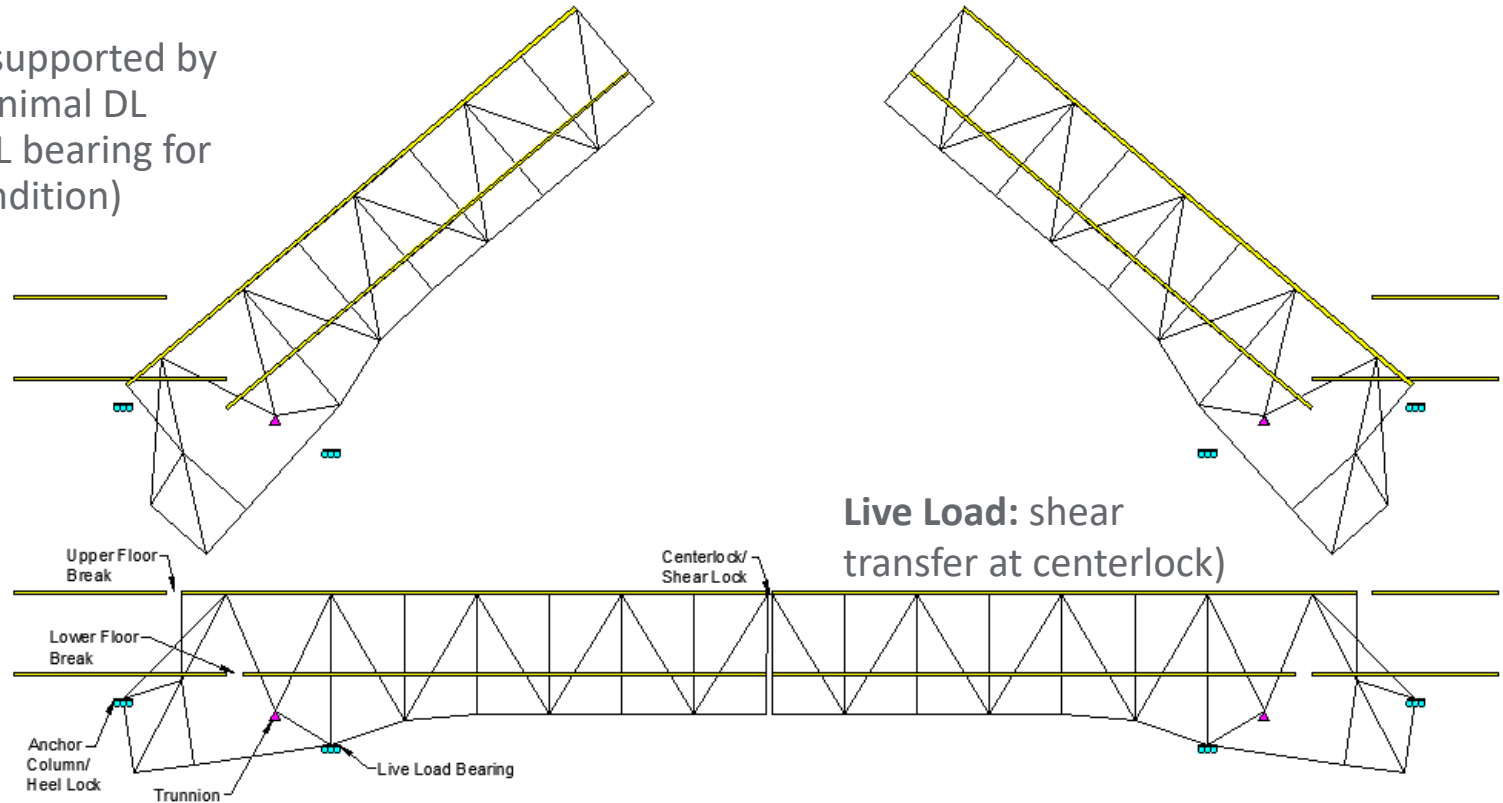
HS 20 Design Live Load for Lower Level Floor System constructed in 1994 to replace original lower level framing

Lake St. Bascule

Load Stages

Dead Load: supported by trunnion (minimal DL reaction at LL bearing for balanced condition)

Live Load: supported by trunnion, LL bearing & heel lock



2 Stage Load Model: Dead load force overrides are required for AASHTOWare analysis since dead load and live load have different support conditions

Lake St. Bascule

Live Load Supports

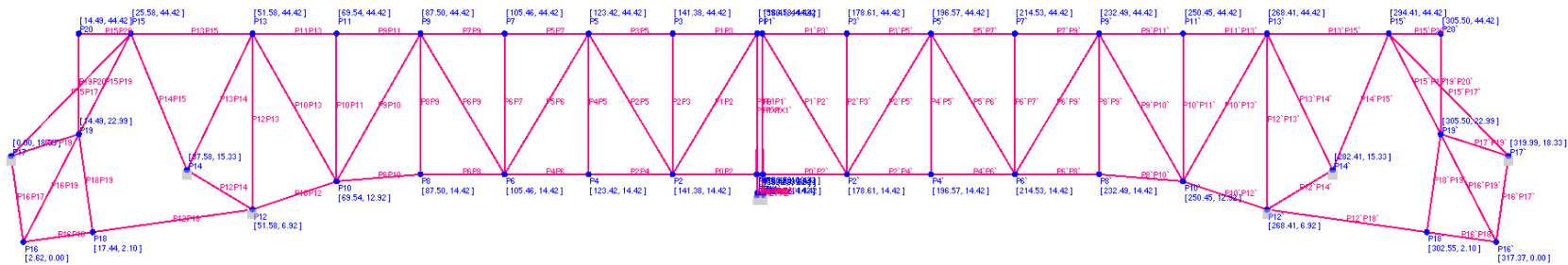
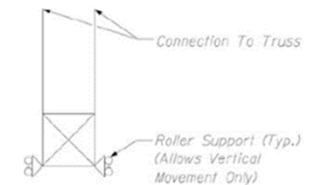
Vehicle and Pedestrian Load on Lower Level & CTA Loading on Upper Level

Supports

- Pin Support at Trunnion
- Roller Support at Live Load Bearing (verified no uplift)
- Roller Support at Heel Lock

Centerlock

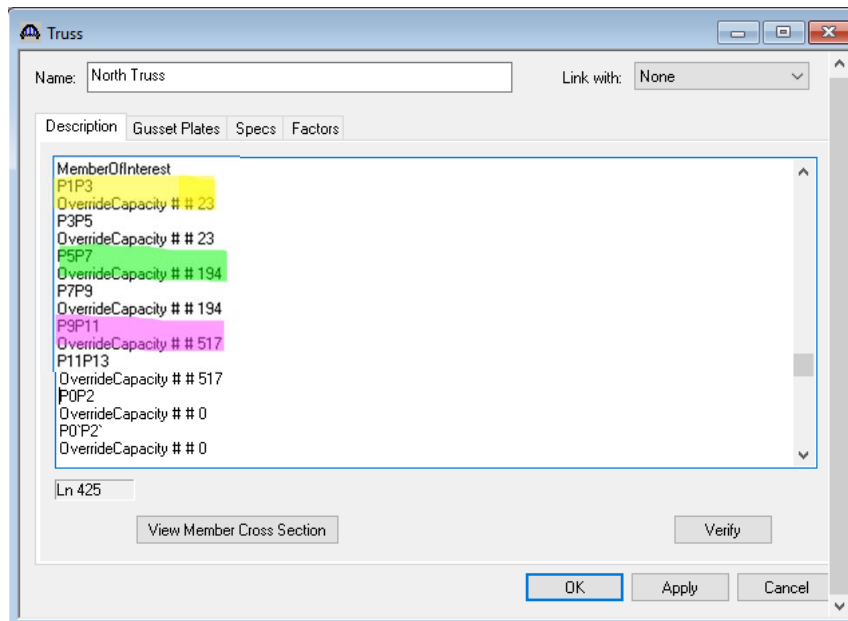
- Centerlock transfers shear
- Does not transfer moment or axial load



Lake St. Bascule

Dead Load Analysis

Input of Dead Load Force Overrides



Verification of Dead Load Forces used for Analysis

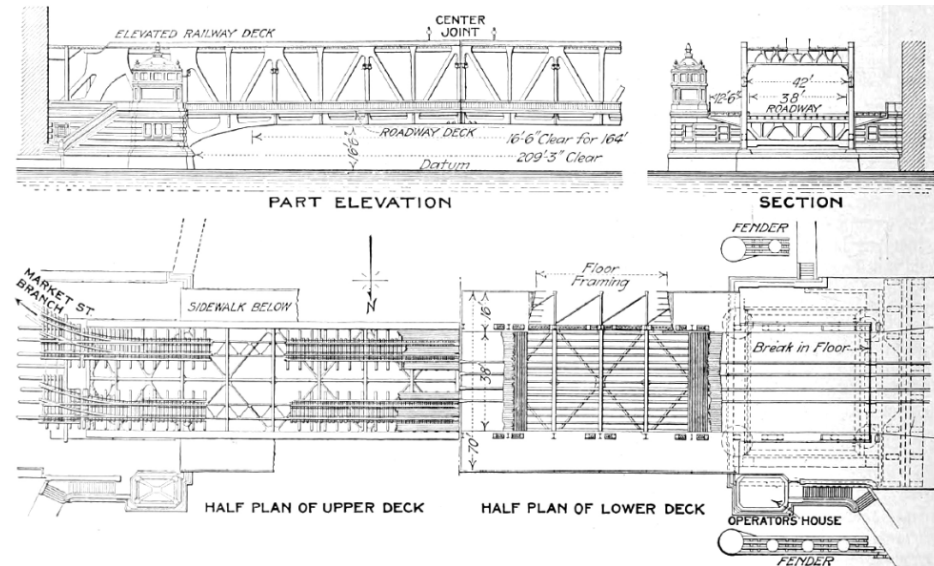
Member	Truss Element	DL Force (kip)	LL Force				Capacity	
			Comp. (kip)	IF	Tens. (kip)	IF	Comp. (kip)	Tens. (kip)
P10P12	Lower-Chord	-688.00	-153.79	1.28	0.11	1.21	-2629.98	3142.80
P8P10	Lower-Chord	-346.00	-94.84	1.28	0.97	1.21	-1507.39	1805.40
P6P8	Lower-Chord	-345.00	-94.31	1.28	1.02	1.21	-1884.35	2256.60
P4P6	Lower-Chord	-88.00	-34.64	1.28	2.37	1.21	-553.48	660.00
P2P4	Lower-Chord	-88.00	-34.38	1.28	2.44	1.21	-553.48	660.00
P0P2	Lower-Chord		-0.21	1.28	0.06	1.21	-473.06	564.00
P10`P12`	Lower-Chord	-688.00	-153.49	1.28	0.11	1.21	-2629.98	3142.80
P8`P10`	Lower-Chord	-346.00	-94.59	1.28	0.97	1.21	-1507.39	1805.40
P6`P8`	Lower-Chord	-345.00	-94.05	1.28	1.06	1.21	-1507.58	1805.40
P4`P6`	Lower-Chord	-88.00	-34.53	1.28	2.40	1.21	-553.48	660.00
P2`P4`	Lower-Chord	-88.00	-34.28	1.28	2.47	1.21	-553.48	660.00
P0`P2`	Lower-Chord		-0.21	1.28	0.06	1.21	-473.06	564.00
P12P18	Lower-Chord	-413.00	-49.96	1.30	3.48	1.30	-2439.74	2502.90
P12`P18`	Lower-Chord	-413.00	-49.87	1.30	3.48	1.30	-2439.74	2502.90
P16P18	Lower-Chord	-459.00	-47.11	1.30	3.77	1.30	-2000.00	2000.00
P16`P18`	Lower-Chord	-459.00	-47.02	1.30	3.77	1.30	-2000.00	2000.00
P11P13	Upper-Chord	517.00	-0.31	1.21	128.82	1.28	-2770.65	2695.80
P9P11	Upper-Chord	517.00	-0.41	1.21	128.55	1.28	-2770.65	2695.80
P7P9	Upper-Chord	194.00	-2.27	1.21	61.46	1.28	-1204.46	1162.20
P5P7	Upper-Chord	194.00	-2.30	1.21	61.18	1.28	-1204.46	1162.20
P3P5	Upper-Chord	23.00	-2.06	1.21	14.55	1.28	-1030.56	995.40
P1P3	Upper-Chord	23.00	-2.12	1.21	14.25	1.28	-927.32	895.80



Lake St. Bascule

Combined Upper and Lower Level Live Load

- Upper Level – CTA Trains
- Lower Level – Vehicles
- Upper and loads analyzed on separate superstructure alternatives in BrR
- Truss loads combined to calculate rating factors for concurrent upper and lower level live loading



Combined Opr. RF (Comp.)	Combined Opr. RF (Tension)
1.31	99.00
1.50	30.88

LL Force (CTA)				LLDF (CTA)
Comp. (kip)	IF	Tens. (kip)	IF	
-190.89	1.28	4.62	1.21	1
-50.51	1.28	7.48	1.21	1

Member	Truss Element	DL Force (kip)	LL Force				Capacity	
			Comp. (kip)	IF	Tens. (kip)	IF	Comp. (kip)	Tens. (kip)
P6P8	Lower-Chord	-345	-139.71	1.28	2.03	1.21	-1507.58	1805.4
P4P6	Lower-Chord	-88	-59.48	1.28	4.03	1.21	-553.48	660

Roosevelt Rd. Bascule

Deck Truss Double Leaf Trunnion Type Bascule



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Roosevelt Rd. Bascule Original Construction

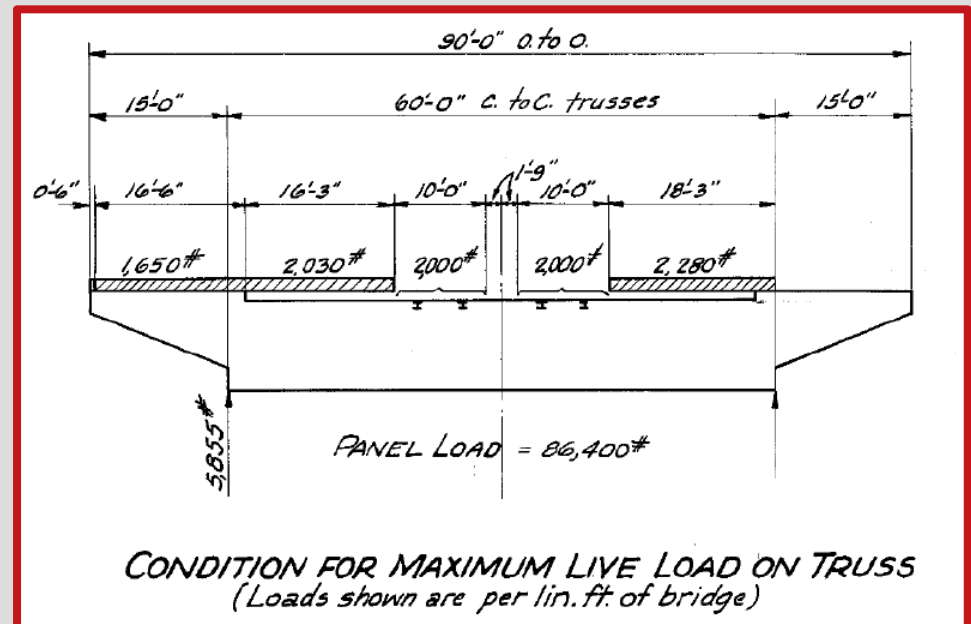
Construction/Repair History

- 1928: Original Construction
- 1973-1975: Isolated repairs of bascule truss, deck replacement and rebalancing
- 1994: Major rehabilitation including replacement of floorbeams, stringers and lateral bracing in movable span and rebalancing
- 1,164 existing plan sheets

Geometry

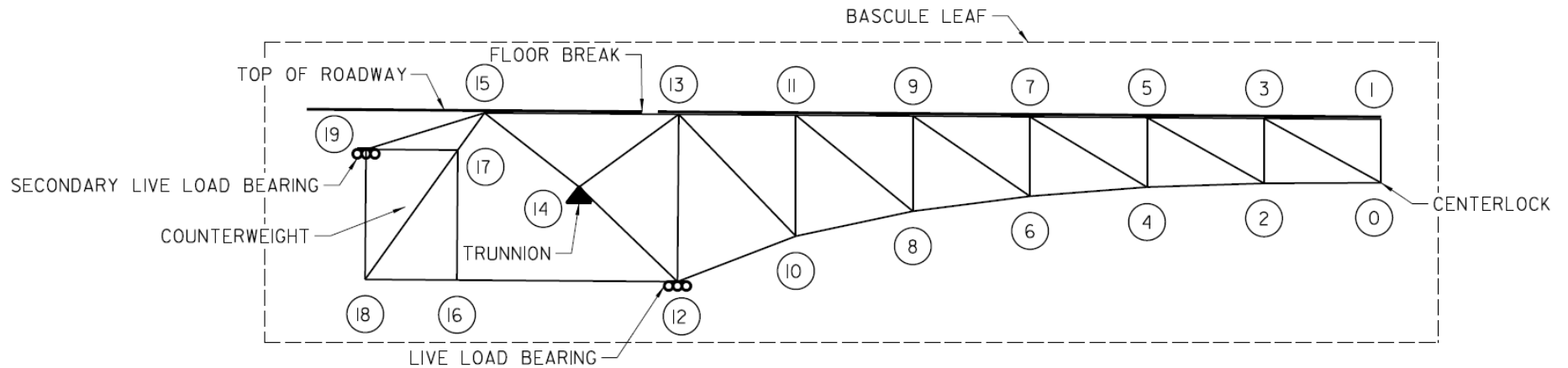
- 204'-0" CL to CL Trunnions
- 28'-9" End Span (each side)

• Design Load



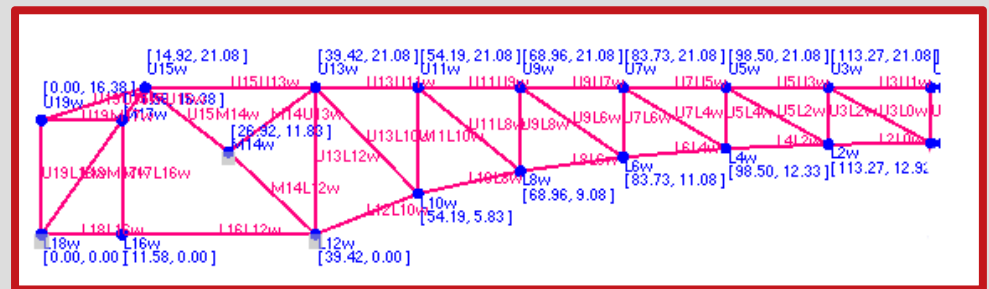
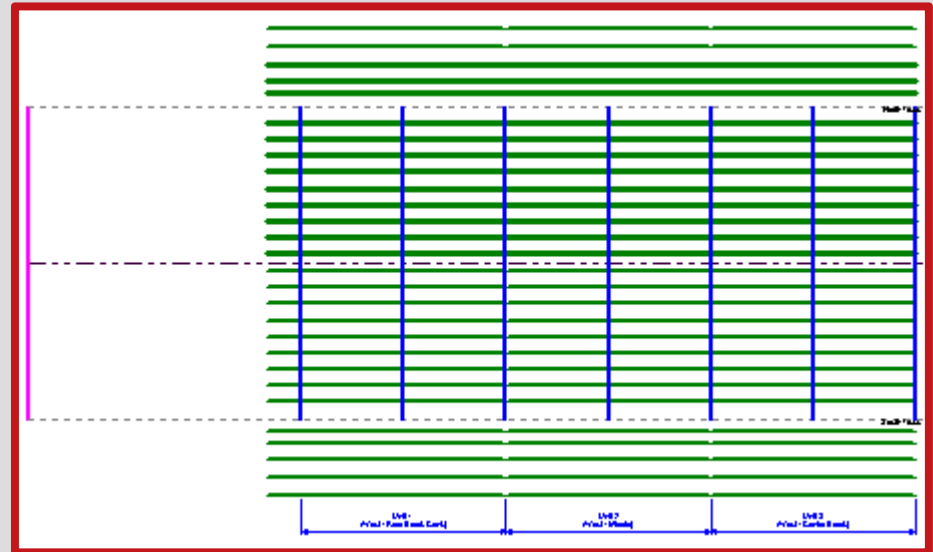
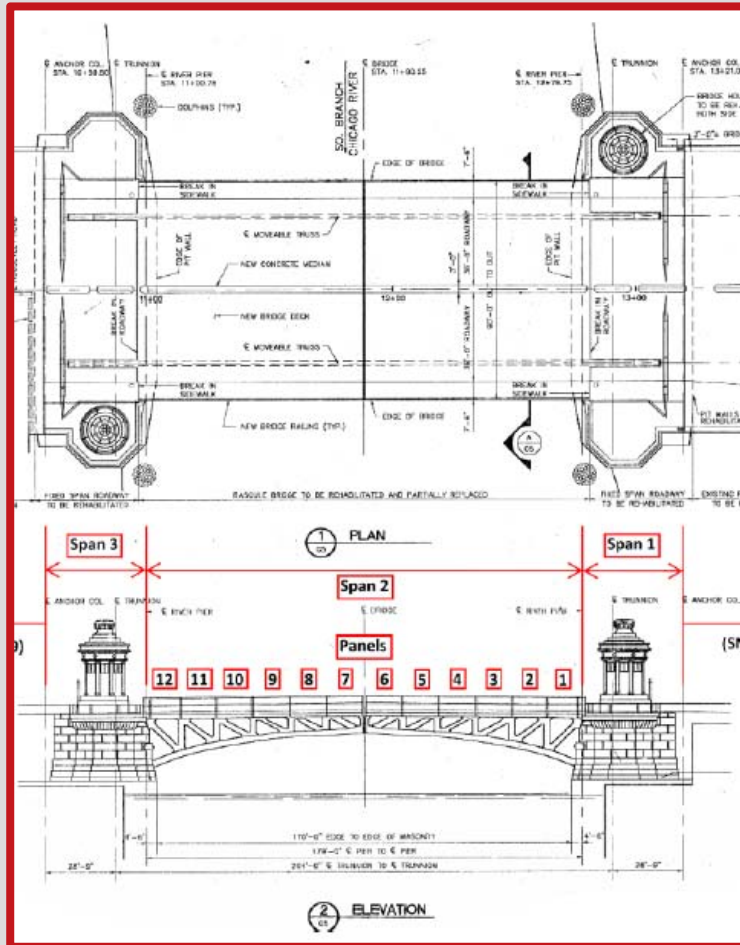
Roosevelt Rd. Bascule

Bascule Structure Components and Supports



Roosevelt Rd. Bascule

Plan and Elevation

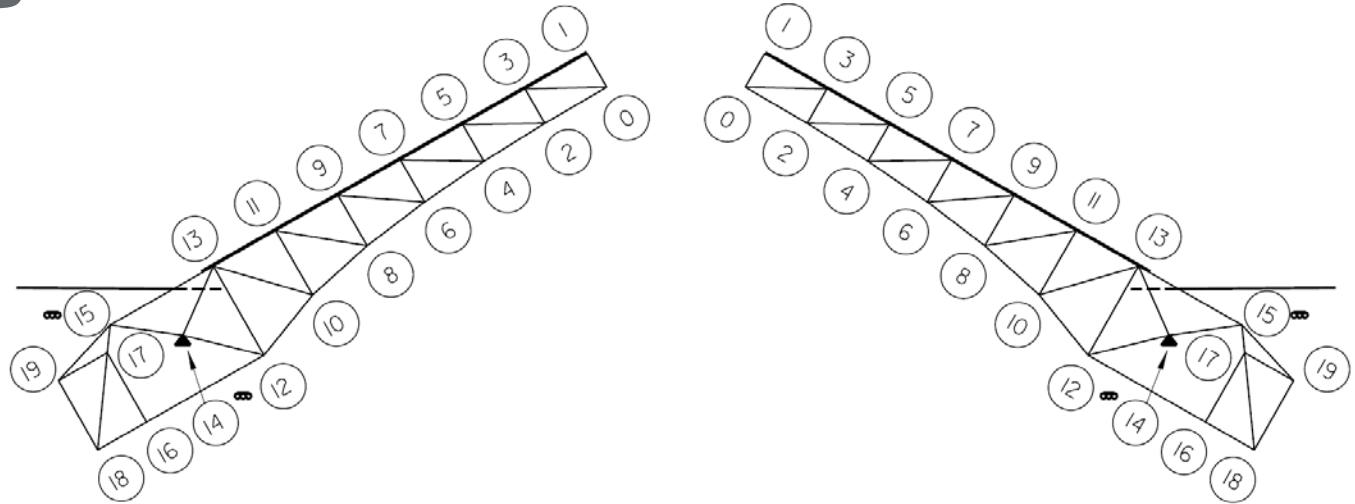


Roosevelt Rd. Bascule

Load Stages

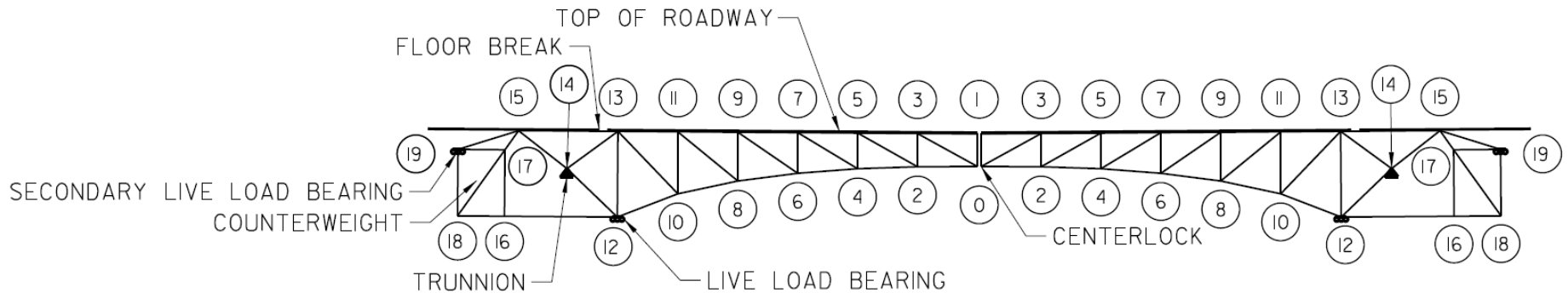
BRIDGE OPEN

Dead Load + Wind Load



BRIDGE CLOSED

Dead Load + Live Load (Including Pedestrian Load)



Roosevelt Rd. Bascule

Dead Load Forces

Dead Loads

- Truss member self weight
- Floorbeams
- Stringers
- Cantilever sidewalks
- Lateral Bracing
- Deck (Steel Grating)
- Counterweight
- Inspection Walkway

ALL WEIGHTS ARE FOR ONE LEAF																
ITEM	WEIGHT	X' IN FT.		X' MOMENT IN FT. LBS.		Y' IN INCHES		Y' MOMENT IN INCH LBS.		ITEM	WEIGHT	X' IN FT.		Y' IN INCHES		
		+	-	+	-	+	-	+	-							
0	880.5	101.25		890400		15.0		132100		Brought Forward	876465					
1	4095	101.25		414100		111.0		454500		Crwth Box - Front Truss	62145	-5.333		952900	-391.5	50942600
2	1451.8	86.354		1255900		13.0		188700		- - Top - -	414.80	-23.11		917100	59444	2259200
3	977.5	86.354		844100		111.0		102500		- - Rear Girder	41870	-24.917		112700	-5732	2400000
4	1754.5	115.83		1241400		4.0		104100		- - Bott. - -	192.25	-21.12		406000	-66.25	3004000
5	1285.0	71.583		919800		111.0		124400		- - Cross Frames	278.40	-18.34		510800	-66.0	1282800
6	1970.5	56.872		1119400		-9.0		177300		Rack & Bolts	23500	-11.6		272600	-38.5	904800
7	1945.0	56.872		1133400		111.0		2214400		Trunnion & Collars	26000	0		0	0	0
8	2714.0	42.842		1141000		-33.0		895800		Rear Bumper Casting	1870	-5.0		54200	-50.0	9350
9	2183.5	42.842		918000		111.0		2423700		Br in Fl. Casting - Front	6700	10.62		680900	114.0	777200
10	26700	23271		728100		-72.0		1922400		- - - - Rear	21190	11.25		238400	110.0	2330900
11	2878.5	2727		785000		111.0		3195100		Gussets for Rear	68025	-14.0		1240500	18.5	1639200
12	5824.0	12.5		729500		-42.0		8287100		Center Island	11685	55.37		647000	123.0	1437300
13	7038.5	12.5		874800		111.0		7812700		Center Lock Platform	7490	94.0		719000	4.0	4420
14	5570	0		0		0		0		Cent Lock Mchry & Castings	12200	95.4		1164800	22.7	216900
15	40240	-12.0		482900		111.0		4466600		Live Load Casting at @	3460	12.5		43300	-149.0	584700
16	15210	-15.333		233200		-42.0		2159800		Strut at @	7740	12.5		96900	-142.0	1100500
17	14725	-15.333		224800		54.464		802000		@ Total for One Leaf, for Initial Loading	1279475			36834800	702.6200	59708200
18	13755	-24.97		370200		-42.0		1933200			23913	-24928600		23903	-20582900	
19	8485	-26.97		233800		54.464		473000								
Floor Beam 0-0	24440	101.25		2475500		64.6		1581400		Edge Angles	3980	57.1		127300	117.0	465700
- - 2-2	22440	86.354		1937800		60.0		1244600		Trolley Support at @	4250	61.23		429800	245.0	104300
- - 4-4	22730	71.883		1427100		46.7		1288800		- - - - @	3650	27.27		107700	245.0	967100
- - 6-6	21235	56.872		1208700		51.7		1049900		Ramp & Sunk Floor, Etc	308100	56.0		17253600	114.8	35986100
- - 8-8	18780	42.842		789500		41.9		786900		Concrete - Top Chord - Rear	6600	3.1		20510	127.75	843200
- - 10-10	24670	27271		672800		44.6		1100300		Concrete at Gussets at R	14000	0.21		3100	41.5	605900
- - 12-12	38730	14875		576100		41.0		1587400								
Horizontal Truss 12-10	3440	21.75		118800		86.5		472300		@ Total for Left Leaf Complete	1620955			54896800	702.6200	94618100
Latticed Strut 12-14	11670	6.04		70500		-68.5		794400			29592	-247870600		43488	-170492800	
Sidewalk Brackets 14-11	14535	69.97		1061500		87.0		1438500		Item @ with 2% increase	1652325			5594700	7166700	101610500
- - 13-8	2755	48.975		41000		87.0		239700			9852	-448828000		43488	-171902200	
- - Laterals 14-11	1240	64.97		87300		113.0		153700		Item @ with 4% increase	1685795			51042600	7307200	103602900
- - 11-10.5	270	21.073		5700		113.0		30580			29592	-44785400		43488	-173312400	
Curb 12-6 Brackets	5345	36.92		303700		129.0		689500								
Bottom Laterals 0-6-2	3930	93.74		368400		14.0		35000								
- - 2-6-4	3755	29748		296500		9.5		35700		Item @ with 2% increase	1801265			53011000	7026200	94296300
- - 4-6-6	3755	24197		241100		-1.5		5600		Cent Lock Platform - W Leaf	1500	99.6		179300	11.0	14920
- - 6-6-8	4420	44427		228400		-21.0		97000		Cent Lock Mchry - W Leaf	5800	100.6		382300	15.0	57000
- - 8-6-10	7480	34464		274600		-53.5		419000		@ Total for W Leaf Complete	1606865			53572600	7026200	94373200
- - 10-6-12	5960	18895		118100		-107.0		635600								
Stringers 14-11	100450	14.17		6448600		100.5		10095200		Item @ with 2% decrease	1574730			52501100	6883700	97385600
- - 14-6 Br in Floor	26500	15.77		523900		98.0		2537000						45615400		14920
Rail Castings	13810	35.8		770600		104.0		1436200								
Tie Plates Rail Break	1145	13.9		16400		108.25		129400								
Carry Forward	876465			33262500		1545900		50942600		17352000						

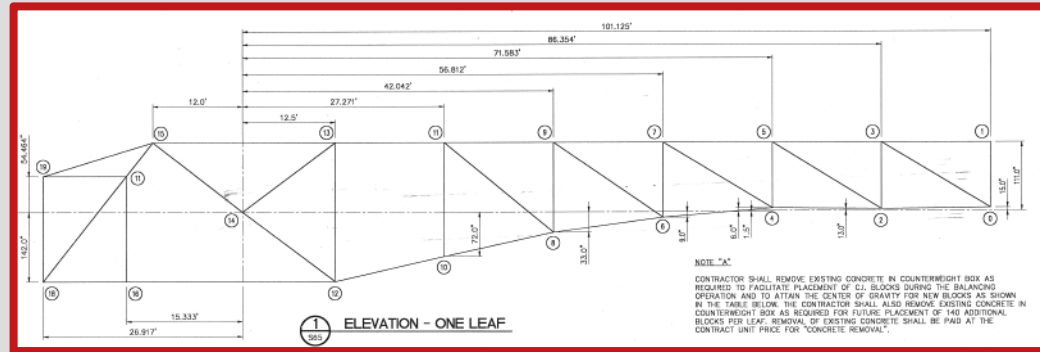
* Crwth Pockets to provide adjustment from 2% below to 4% above moment required to balance leaf



Roosevelt Rd. Bascule

Dead Load Balancing

- 1928: Original Balancing
- 1973: Rebalancing
- 1994: Rebalancing



C.G. AND MOMENT - FIGURES ARE PER LEAF									
BALANCING CALCULATIONS									
ITEM	NO.	UNIT WEIGHT	VL PER LIN. FT.	LENGTH	TOTAL WEIGHT PER LEAF (LBS)	X' FEET	MOMENT (FT.-LBS.)	INCHES	MOMENT (IN.-LBS.)
MATERIALS REMOVED									
1	FB 0-0	1			24,480.00	101.13	2,475,540.00	84.60	1,982,408.00
2	FB 2-2	1			22,440.00	86.35	1,937,783.75	80.00	1,346,400.00
3	FR 4-4	1			22,730.00	71.58	1,627,081.50	56.70	1,288,791.00
4	FR 6-6	1			21,275.00	56.81	1,205,675.30	51.70	1,039,917.50
5	FR 8-8	1			18,780.00	42.04	789,548.75	41.90	789,882.00
6	FR 10-10	1			24,670.00	27.27	672,775.57	44.60	1,100,282.00
7	FR 12-12	1			38,730.00	14.88	576,108.75	41.00	1,587,930.00
8	BOTTOM STRUT	1			6,154.00	12.50	76,920.00	-142.00	-873,868.00
LATRAL BRACING:									
9	0-0 TO 2-2	1			3,930.00	93.74	368,388.20	14.00	55,020.00
10	2-2 TO 4-4	1			3,755.00	79.97	298,524.84	9.50	39,872.50
11	4-4 TO 6-6	1			3,755.00	64.20	241,058.74	6.00	22,330.00
12	6-6 TO 8-8	1			4,820.00	49.43	238,352.74	-21.00	-97,020.00
13	8-8 TO 10-10	1			7,980.00	34.66	276,334.66	-52.50	-418,950.00
14	10-10 TO 12-12	1			5,940.00	19.89	118,116.30	-107.00	-635,380.00
15	WT 15X 49.5 ON FB'S	0			14,040.00	58.87	798,454.80	115.80	1,625,832.00
ROADWAY STRINGERS:									
16	FB 0-0 TO 10-10 (W/BOXES)	18			104,370.00	84.20	8,720,564.00	119.90	12,513,863.00
17	FB 10-10 TO REAR BREAK (W/4X 6)	19			34,040.00	18.44	627,697.80	121.20	4,123,848.00
18	ROWY. STRINGER SUPPORTS (W REAR BREAK)	6			6,740.00	19.88	133,991.2*	110.60	745,444.00*
19	ORIGINAL STRINGERS, FB10-10 TO REAR BREAK				26,500.00	19.77	523,905.00	98.00	2,597,000.00
20	ORIGINAL STRINGERS, STUBS & FLOORBEAMS				6,500.00	64.20	417,300.00	27.52	178,886.50
21	5" CONC. FILLED ROADWAY DECK		58.0 PSF		141,580.00	62.37*	8,830,344.60*	130.50	18,490,348.00
22	5" CONC. ROADWAY DECK		19.1 PSF		86,870.00	52.07	4,523,320.50*	130.60	11,359,222.00*
23	REAR BREAK WELDMENTS:				148.66*	74.80*	11,120.00*	9.97	110,977.60*
24	CENTER BREAK WELDMENTS:				119.89*	75.40*	9,040.00*	101.32	919,633.80*
25	CENTER BREAK WELDMENT SUPPORT	1			3,210.00	101.17	324,755.70*	116.30	373,323.00*
26	ROADWAY MEDIAN				335.50	90.50	30,360.00	55.82	1,694,695.20
27	FENCE (VIB. SUPPORTS)				43.40	180.00	7,820.00	25.36	422,915.20*
28	STEEL CURB (INCL. GROUT)				42.30	184.00	7,780.00	56.10	436,458.00
					TOTALS (EAST LEAF)	689,209.00*	57,364,748.63*		67,818,793.50*
					TOTALS (WEST LEAF)	689,209.00*	57,364,748.63*		67,818,793.50*

C.G. AND MOMENT - FIGURES ARE PER LEAF										
BALANCING CALCULATIONS										
ITEM	NO.	UNIT WEIGHT	VL PER LIN. FT.	LENGTH	TOTAL WEIGHT PER LEAF (LBS)	X' FEET	MOMENT (FT.-LBS.)	INCHES	MOMENT (IN.-LBS.)	
MATERIALS ADDED										
1	FB 0-0	1			15,000.00	101.25	1,500,000.00	78.32	1,182,000.00	
2	FB 2-2	1			10,580.00	86.35	915,331.00	75.64	907,471.20	
3	FR 4-4	1			15,080.00	71.58	1,079,332.40	60.88	1,055,892.40	
4	FR 6-6	1			16,230.00	56.81	922,026.30	55.74	905,060.00	
5	FR 8-8	1			15,830.00	42.04	665,173.20	54.11	679,488.30	
6	FR 10-10	1			18,780.00	27.27	513,735.30	35.97	513,840.30	
7	FR 12-12	1			29,880.00	14.88	443,880.00	33.50	412,480.00	
8	BOTTOM STRUT	1			6,154.00	12.50	76,920.00	-142.00	-873,868.00	
LATRAL BRACING:										
9	0-0 TO 2-2	1			5,618.00	93.74	526,631.30	13.25	74,438.00	
10	2-2 TO 4-4	1			5,618.00	79.97	443,700.84	8.75	49,182.75	
11	4-4 TO 6-6	1			5,618.00	64.20	360,268.74	6.35	28,511.40	
12	6-6 TO 8-8	1			5,627.76	49.43	278,180.18*	-21.75	-122,403.78	
13	8-8 TO 10-10	1			5,664.19	34.66	195,627.64*	-53.35	-309,251.12	
14	10-10 TO 12-12	1			5,702.88	19.89	113,423.68*	-107.75	-614,450.84	
ROADWAY STRINGERS:										
15	FB 0-0 TO REAR BREAK	24			21,703.80*	78.50*	1,702,889.1*	141.48	6,273,888.88*	
16	FB 10-10 TO REAR BREAK	24			61,356.00*	29.75*	1,829,429.5*	121.42	7,444,262.84*	
17	5" CONC. FILLED ROADWAY DECK		49.3 (PSF)		333,586.00	55.01*	18,382,532.56*	133.67	44,801,134.22*	
18	ROADWAY DECK CURBS				99,659.50*	53.75*	5,356,872.88*	131.31*	7,833,784.91*	
CENTER BREAK WELDMENTS:										
19	THRU ROADWAY (EAST)				11,041.00	101.44	1,121,278.08	133.67	1,478,880.47	
20	THRU ROADWAY (WEST)				163.50	101.48	16,588.71	143.00	23,350.50	
21	THRU SIDEWALK (EAST)				852.10	101.48	86,389.44	143.00	109,424.46	
22	THRU SIDEWALK (WEST)				13,528.00	9.98	135,009.44	129.40	1,790,520.00	
23	REAR BREAK WELDMENT				30,244.00	39.62	1,204,495.68	135.00	4,123,450.00	
24	ROADWAY MEDIAN				23,063.00	95.13	2,211,463.18*	160.20	3,706,224.10	
INSPECTOR PLATFORM:										
25	EAST AND WEST LONGITUDINAL CENTER LINE				4,428.00	47.75	209,043.75	-1.00	-5,428.00	
26	WEST TRANSVERSE BRACKETS				4,788.00	89.00	426,132.00	37.25	178,353.00	
27	WEST BRACKETS				1,932.00	93.00	179,679.00	37.25	71,987.00	
28	WEST BRACKETS				1,440.00	34.41	49,510.40	133.87	182,483.80	
29	SCAFFOLD WEST LEAF				860.00	45.85	39,232.00	85.87	128,583.50	
30	LATRAL BRACING (TOP CHORD BETWEEN 10-10 AND 12-12)				9,804.80	21.15*	207,373.64*	108.47*	1,063,537.50*	
31	ROADWAY CURB	2			6,918.75*	56.10*	388,141.88*	139.29*	5,087,071.50*	
					TOTALS (EAST LEAF)	799,878.37*	43,038,480.71*		88,423,898.25*	
					TOTALS (WEST LEAF)	799,878.37*	43,038,480.71*		88,402,918.77*	
					TOTAL WEIGHT (EAST LEAF)	100,469.37*				
					TOTAL WEIGHT (WEST LEAF)	100,293.37*				
					UNBALANCED X MOMENT (EAST LEAF) IN FT.-LBS.		5,671,712.08*			
					UNBALANCED X MOMENT (WEST LEAF) IN IN.-LBS.		5,673,504.09*			
					COUNTERWEIGHT ADDED (EAST SIDE)	695*	450 PSF	312,750*	18.14*	5,670,157.00*
					COUNTERWEIGHT ADDED (WEST SIDE)	695*	450 PSF	312,750*	18.14*	5,673,285.00*



Roosevelt Rd. Bascule

Truss Line Input

- Section Properties calculated in Excel, accounting for section loss as required
 - Ag, An, Material, Ix, Iy, Sx, Sy
 - Section Loss (if applicable)

MEMBER: 1-3 DESIGNATES USER INPUT

$d_s - 6 \times d_s = 10.00$
 $21600 - 22 \times 2 = 19.25$

33.69 /-3

No.	Section	X _c (in)	Y _c (in)	A _g (in ²)	I _x (in ⁴)	I _y (in ⁴)	d _x (in)	d _y (in)	A _x X _c (in ³)	A _y Y _c (in ³)	I _x +A _x d _x ² (in ⁴)	I _y +A _y d _y ² (in ⁴)
1	L6x4x3/8	3.06	1.94	3.61	13.47	4.90	12.25	9.31	11.05	7.00	326.37	546.85
2	L6x4x3/8	3.06	20.56	3.61	13.47	4.90	12.25	-9.31	11.05	74.22	326.37	546.85
3	L6x4x3/8	27.57	1.94	3.61	13.47	4.90	-12.25	9.31	99.51	7.00	326.37	546.85
4	L6x4x3/8	27.57	20.56	3.61	13.47	4.90	-12.25	-9.31	99.51	74.22	326.37	546.85
5	22x7/16	4.22	11.25	9.63	388.21	0.15	11.09	0.00	40.61	108.28	388.21	1184.71
6	22x7/16	26.41	11.25	9.63	388.21	0.15	-11.09	0.00	254.16	108.28	388.21	1184.71
7							0.00	0.00	0.00	0.00	0.00	0.00
8							0.00	0.00	0.00	0.00	0.00	0.00
9							0.00	0.00	0.00	0.00	0.00	0.00
10							0.00	0.00	0.00	0.00	0.00	0.00

I = 33.69 515.88 379.01 2081.90 4556.82

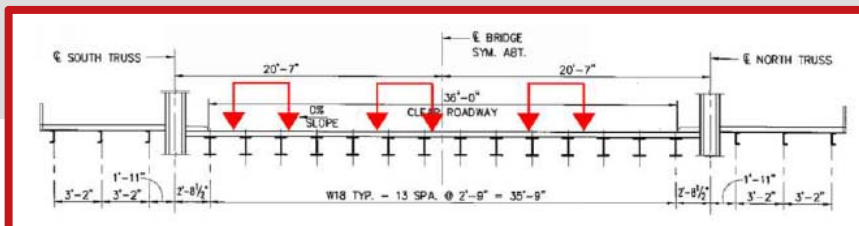
GROSS PROPERTIES

X̄ = 15.31 in r_x = 7.86 in
 Ȳ = 11.25 in r_y = 11.63 in
 A_g = 33.69 in² Weight = 0.11 k/ft
 I_x = 2081.90 in⁴ S_{x,top} = 185.06 in³
 I_y = 4556.82 in⁴ S_{x,bot} = 185.06 in³
 Depth = 22.50 in S_y = 297.59 in³

No. Holes	Width (in)	Thickness (in)	Area (in ²)
8	1.13	0.44	3.94
8	1.13	0.38	3.38
			0.00
			0.00

A_{gross} = 7.31 in²

- Live Load Distribution Factors



```
Truss "North Truss"
Unit
Force kips
Length ft
Properties in
DefaultSysUnitType US
DefaultStructSteel "1905 to 1936"
DefaultEndConnection
Riveted
//DefaultMemConnection command not currently in use
MaterialType
steel1927 = "1905 to 1936"
Gr36Steel = "Grade 36"
Gr50Steel = "Grade 50"
//Member Cross Sections
//NonDetailed = <section name>
//gross area, net area, material type, Ix, Iy, Sx, Sy
MemberCrossSection //top chord
NonDetailed = section31
33.69 26.38 steel1927 2081.90 4556.82 185.06 297.59
NonDetailed = section53
33.69 26.38 steel1927 2081.90 4556.82 185.06 297.59
```

```
PanelPoint
U19w Upper 0 16.375
U15w Upper 14.917 21.083
U13w Upper 39.417 21.083
U11w Upper 54.188 21.083
U9w Upper 68.958 21.083
U7w Upper 83.729 21.083
U5w Upper 98.5 21.083
```

```
//Top chord
U19U15w U19w U15w section1915
U15U13w U15w U13w section1513
U13U11w U13w U11w section1311
U11U9w U11w U9w section119
U9U7w U9w U7w section97
U7U5w U7w U5w section75
U5U3w U5w U3w section53
```

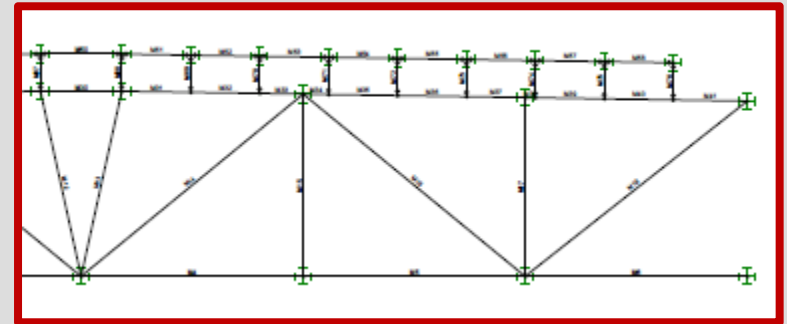
```
PanelPointLoad
L16w DC 0.0 -402
M17w DC 0.0 -402
L18w DC 0.0 -295
U19w DC 0.0 -295
L16e DC 0.0 -402
M17e DC 0.0 -402
L18e DC 0.0 -295
U19e DC 0.0 -295
LLDistribution
OneLane 0.996 0.5
MultiLane 2.528 2.25
```

- Half Deck Line Locations
- Panel Point Loads

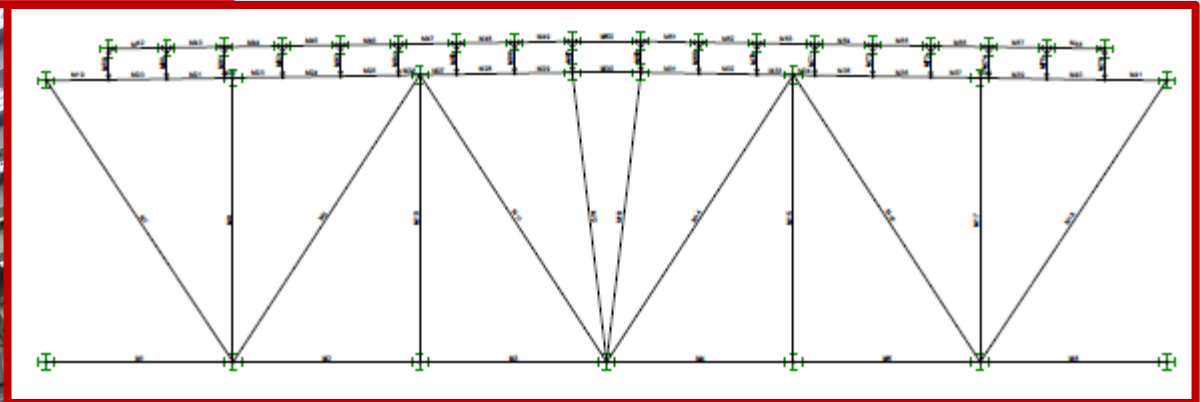
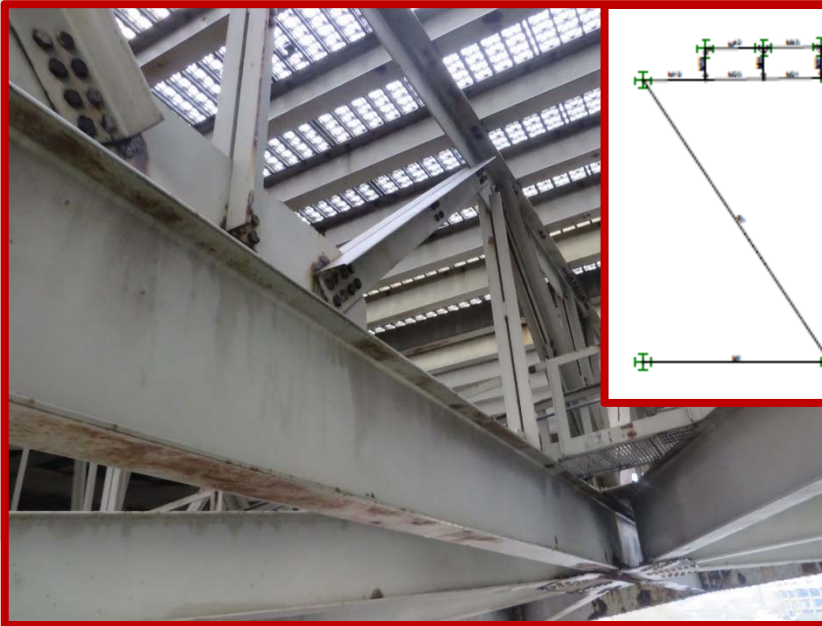


Roosevelt Rd. Bascule Trussed Floorbeams

- *Moveable Span elements mostly rated in BrR*
- *Supplemental analysis independent of BrR for trussed floorbeams*
 - *Floorbeam 2-2*
 - *Floorbeam 10-10*



Floorbeam 2-2



Floorbeam 10-10

Roosevelt Rd. Bascule Carrying Elements

- *Fixed Span elements mostly rated in BrR*
- *Supplemental analysis required outside of BrR for some carrying elements*
 - *Cross Girder (Trunnion Floorbeam)*
 - *Stub Columns*



Cermak Rd. Bascule Scherzer Rolling Lift Bascule Bridge



Cermak Rd. Bascule Bridge Lift



2018 RADBUG MEETING - BOISE, IDAHO



Cermak Rd. Bascule

Original Construction

Construction/Repair History

- 1906: Original Construction
- 1995: Majority of structure replaced as part of rehabilitation
 - Truss top and bottom chords, verticals and diagonals
 - Segmental and Track Girders
 - Anchor Girder and Columns
 - Bumping Girder
 - Floorbeams and Stringers
- 1996: Deck replacement and rebalancing
- 1,064 existing plan sheets

Geometry

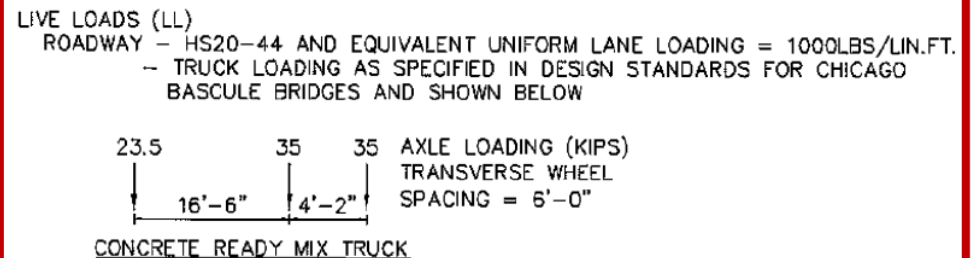
- 216'-0" CL to CL 'First Positions of Roll'
- 29'-4" & 25'-10 1/2" End Spans (each side)

Design Load (Original)

L.L. For Trusses: 3000# per lin ft of bridge occupying 22' in width of roadway and a load of 90# per sq ft for remainder of roadway and sidewalk.

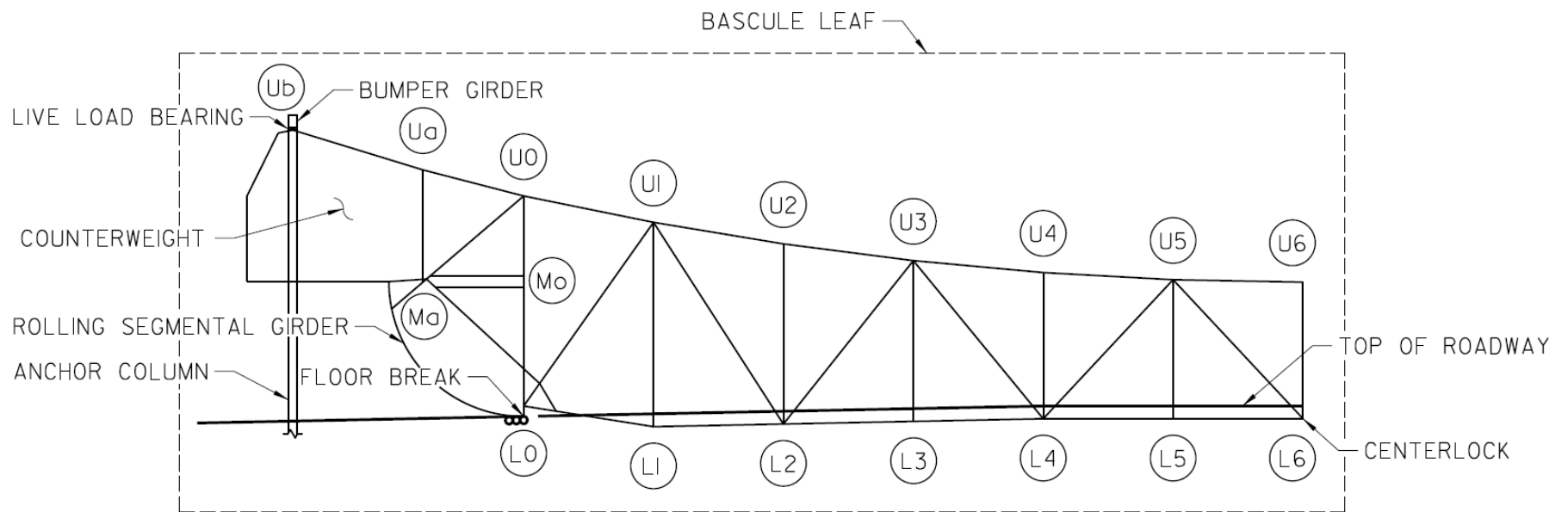
For floor system and hangers: A concentrated load of 24 tons on two axles 10ft. ctrs. on any portion of the roadway, or on each car track for a space of 12' in width for each single track and 22' for double track and in addition a load of 100# per sq ft of remaining surface of roadway and sidewalk.

Design Load (1994 CDOT specifications)

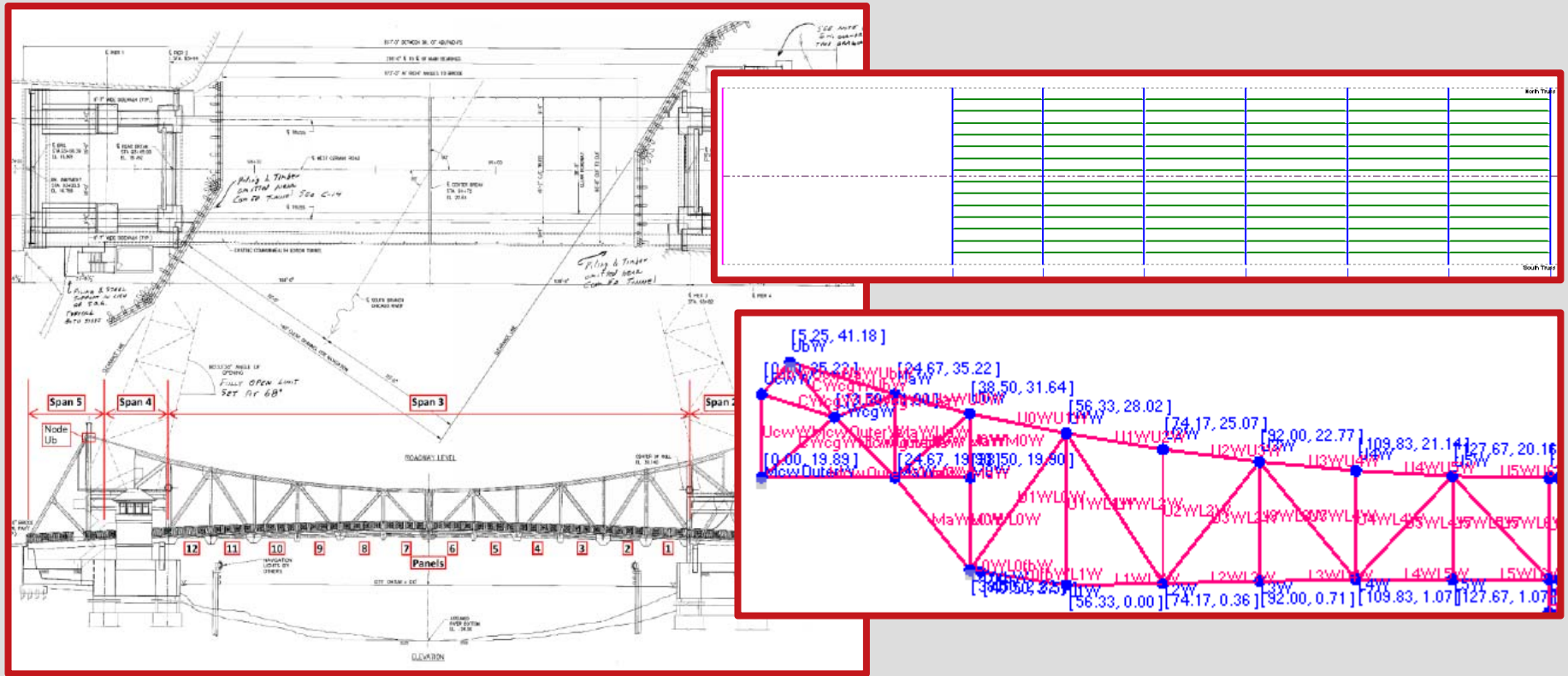


Cermak Rd. Bascule

Bascule Structure Components and Supports

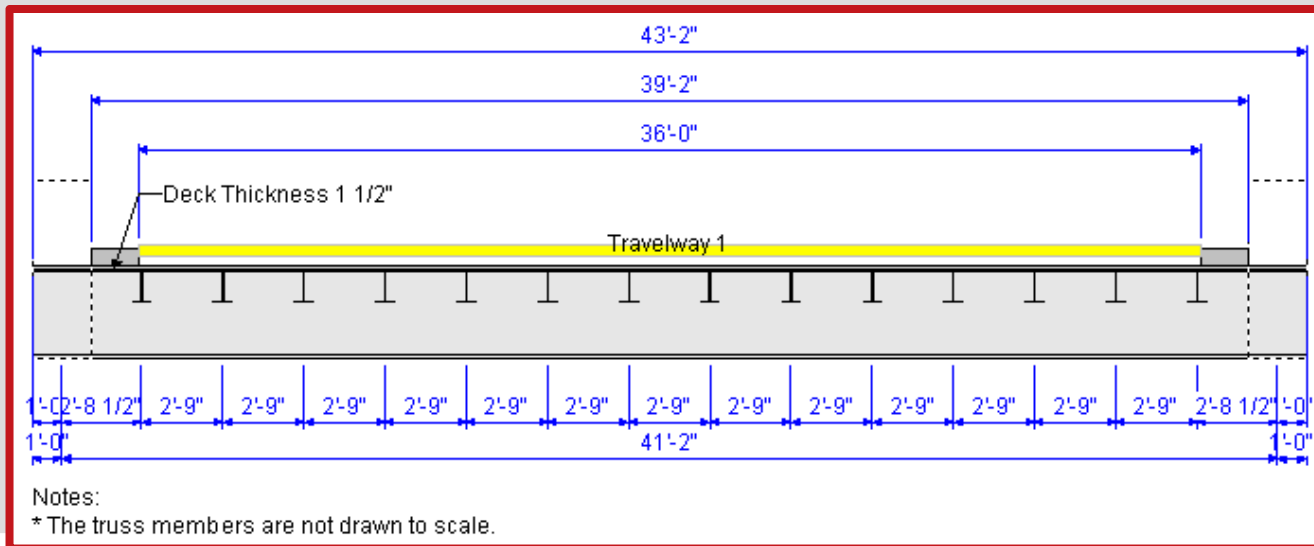
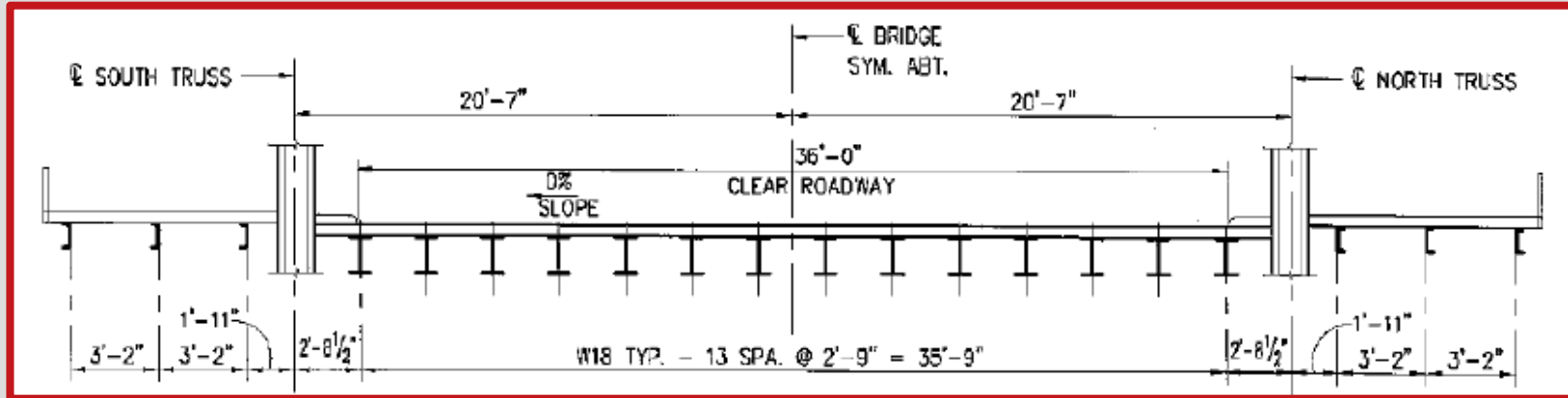


Cermak Rd. Bascule Plan and Elevation



Cermak Rd. Bascule

Cross Section

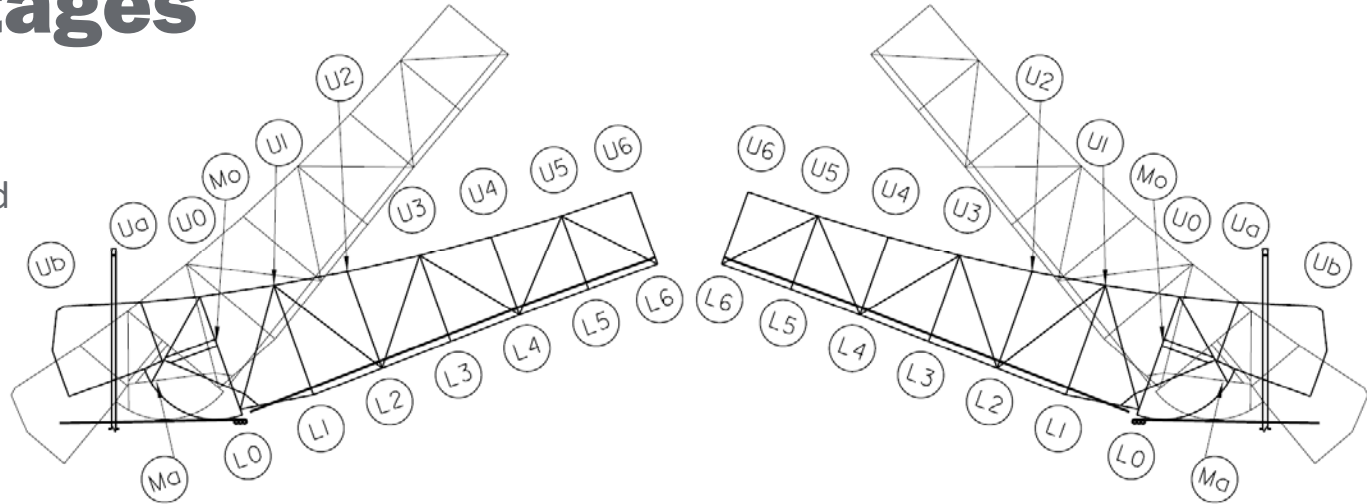


Cermak Rd. Bascule

Load Stages

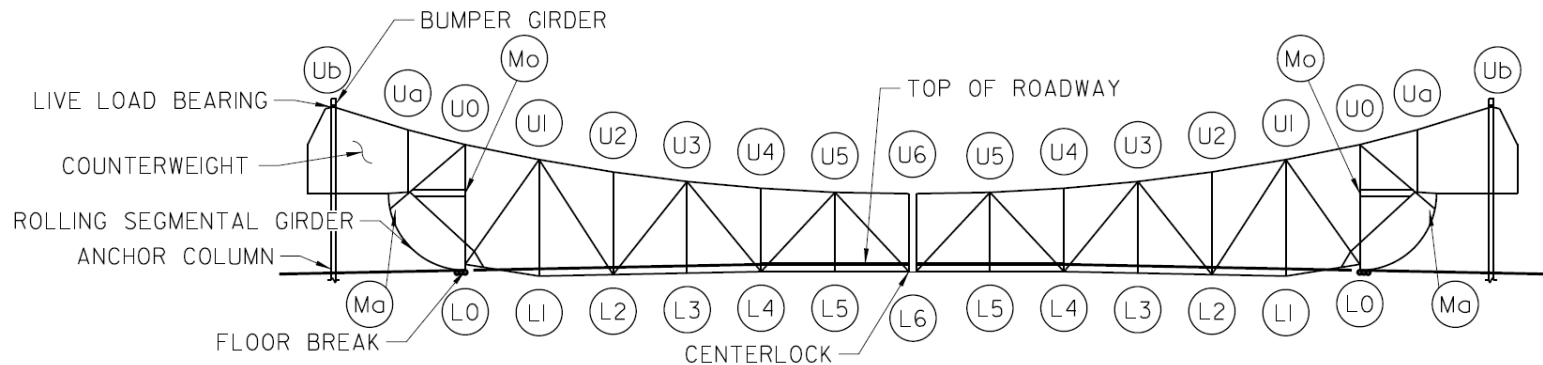
BRIDGE OPEN

Dead Load + Wind Load



BRIDGE CLOSED

Dead Load + Live Load (Including Pedestrian Load)



Cermak Rd. Bascule

Dead Load Forces

Dead Loads

- Truss member self weight
- Floorbeams
- Stringers
- Cantilever sidewalks
- Lateral Bracing
- Deck (Steel Grating)
- Counterweight
- Mechanical Housing

MATERIAL - CARBON STEEL 1" ϕ RIVETS	TOTAL AREA PROVIDED IN SQ. INCHES		MEMBER	CROSS SECTION
	GIR.	NET.		
4L1 - 6" x 6" x 1/2" = 14.00 2 W10s - 22 1/2" = 19.25	33.60		1-3	
4C1 - 6" x 6" x 1/2" = 14.00 - 3.38 = 11.00 2 W10s - 22 1/2" = 19.25 - 9.96 = 15.31	33.60	26.97	3-5	do
2 L3 - 6" x 6" x 1/2" = 27.70 - 6.75 = 21.01 2 W10s - 22 1/2" = 22.00 - 4.50 = 17.50	64.76	50.19	5-7	
4 L1 - 6" x 6" x 1/2" = 31.92 - 3.87 = 24.05 2 W10s - 22 1/2" = 43.00 - 5.85 = 39.15	102.92	81.33	7-9	
2 L1 - 8" x 8" x 1/2" = 31.92 - 3.87 = 24.05 2 W10s - 22 1/2" = 43.00 - 5.85 = 39.15 2 S16s - 16" x 6" = 28.00 - 4.90 = 23.10	136.90	108.38	9-11	
2 L3 - 6" x 6" x 1/2" = 27.70 - 6.75 = 21.01 2 W10s - 22 1/2" = 22.00 - 4.50 = 17.50 2 S16s - 16" x 6" = 28.00 - 4.90 = 23.10 2 L4 - 4" x 4" x 1/2" = 15.96 - 3.99 = 12.03	172.38	135.61	11-13	
4 L1 - 6" x 6" x 1/2" = 31.92 - 3.87 = 24.05 2 W10s - 22 1/2" = 43.00 - 5.85 = 39.15 2 S16s - 16" x 6" = 28.00 - 4.90 = 23.10 2 L4 - 4" x 4" x 1/2" = 15.96 - 3.99 = 12.03	283.88	225.25	13-15	

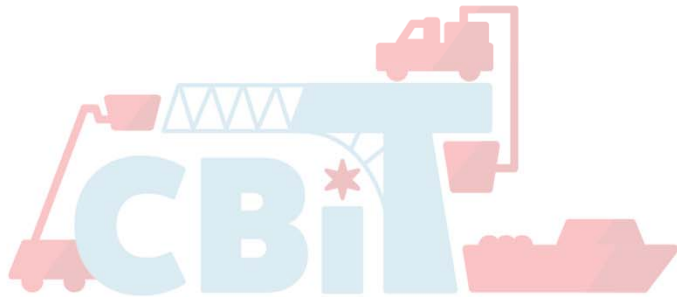


Using BrR To Evaluate Bascule Bridges

2018 RADBUG MEETING – BOISE, IDAHO

AUGUST 7-8, 2018

Thank You!
Questions?



A Joint Venture Teaming of Alfred Benesch &
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